

# **PERFORMANCE-BASED PLASTIC DESIGN**

## **EARTHQUAKE-RESISTANT STEEL STRUCTURES**

**Subhash C. Goel  
Shih-Ho Chao**



## **Performance-Based Plastic Design: Earthquake-Resistant Steel Structures**

ISBN: 978-1-58001-714-5

Project coordinator:	Cindy Rodriguez
Cover design:	Lindsay Seligman
Publications Manager:	Mary Lou Luif
Project editor:	Greg Dickson
Illustrator:	Mike Tamai
Interior design and typesetting:	Yolanda Nickoley

COPYRIGHT 2008



ALL RIGHTS RESERVED. This publication is a copyrighted work owned by the International Code Council. Without advance written permission from the copyright owner, no part of this book may be reproduced, distributed, or transmitted in any form or by any means, including, without limitation, electronic, optical, or mechanical means (by way of example and not limitation, photocopying or recording by or in an information storage and retrieval system). For information on permission to copy material exceeding fair use, please contact: ICC Publications, 4051 W. Flossmoor Rd., Country Club Hills, IL 60478. Phone: 888-ICC-SAFE (422-7233).

Information contained in this work has been obtained by the International Code Council (ICC) from sources believed to be reliable. Neither ICC nor its authors shall be responsible for any errors, omissions, or damages arising out of this information. This work is published with the understanding that ICC and its authors are supplying information but are not attempting to render professional services. If such services are required, the assistance of an appropriate professional should be sought.

First Printing: November 2008

Printed in the United States of America

## Table of Contents

---

<b>FOREWORD . . . . .</b>	<b>ix</b>
<b>ACKNOWLEDGEMENTS . . . . .</b>	<b>xi</b>
<b>CHAPTER 1. INTRODUCTION . . . . .</b>	<b>1</b>
1.1 Current Seismic Design Procedure and Its Weaknesses . . . . .	1
1.2 Performance-Based Plastic Design (PBPD) Method . . . . .	3
<b>CHAPTER 2. PLASTIC DESIGN VERSUS ELASTIC DESIGN . . . . .</b>	<b>7</b>
2.1 Synopsis of Elastic and Plastic Design Methods . . . . .	7
2.2 Illustrative Example . . . . .	9
2.2.1 Elastic Design . . . . .	10
2.2.2 Plastic Design . . . . .	12
<b>CHAPTER 3. PERFORMANCE-BASED PLASTIC DESIGN (PBPD) PROCEDURE . . . . .</b>	<b>17</b>
3.1 General . . . . .	17
3.2 Design Procedure . . . . .	17
3.2.1 Target Yield Mechanism . . . . .	17
3.2.2 Design Lateral Forces . . . . .	19
3.2.3 Design Base Shear . . . . .	20
3.2.4 Design of Designated Yielding Members (DYM <sub>s</sub> ) . . . . .	27
3.2.5 Design of Non-Designated Yielding Members (Non-DYM <sub>s</sub> ) . . . . .	28
<b>CHAPTER 4. MOMENT FRAMES (MF<sub>s</sub>) . . . . .</b>	<b>33</b>
4.1 Overall Design Procedure . . . . .	33
4.2 Design Example . . . . .	33
4.2.1 Building Geometry . . . . .	33

4.2.2 Gravity Loading Criteria . . . . .	33
4.2.3 Gravity Loading Definitions . . . . .	39
4.3 Design Base Shear and Lateral Force Distribution . . . . .	41
4.3.1 Target Yield Mechanism . . . . .	41
4.3.2 Lateral Force Distribution . . . . .	42
4.3.3 Design Base Shear . . . . .	42
4.4 SMF with RBS Connections . . . . .	43
4.4.1 Design of Beams . . . . .	43
4.4.2 Design of Columns . . . . .	49
4.4.2.1 Exterior Column Tree . . . . .	49
4.4.2.2 Interior Column Tree . . . . .	51
4.4.2.3 Column Sections . . . . .	51
4.4.2.4 Check Column Panel Zone Shear Strength . . . . .	54
4.5 SMFs with SW Connections . . . . .	56
4.5.1 Slotted Web Connection . . . . .	56
4.5.2 Design of Beams with SW Connections . . . . .	57
4.5.3 Design of Columns . . . . .	59
4.5.3.1 Exterior Column Tree . . . . .	59
4.5.3.2 Interior Column Tree . . . . .	60
4.5.3.3 Column Sections . . . . .	60
4.5.3.4 Check Column Panel Zone Shear Strength . . . . .	63
4.6 Verification by Nonlinear Analysis . . . . .	64
4.6.1 Frame Designed by Elastic Method . . . . .	64
4.6.2 Nonlinear Analysis Results. . . . .	65

<b>CHAPTER 5. ECCENTRICALLY BRACED FRAMES (EBFs) . . . . .</b>	<b>71</b>
5.1 General . . . . .	71
5.1.1 AISC Design Criteria. . . . .	73
5.2 Overall Design Procedure. . . . .	75
5.2.1 Design Yield Mechanism. . . . .	77
5.2.2 Design of Link Beams . . . . .	78
5.2.3 Design of Non-Yielding Members outside the Links . . . . .	80
5.2.4 Design of Columns with Associated Beam Segments and Braces . . . . .	81
5.3 Design Examples . . . . .	85
5.3.1 Design of 3-Story EBF . . . . .	86
5.3.2 Design of 10-Story EBF . . . . .	93
5.4 Verification by Nonlinear Analysis . . . . .	100
5.4.1 Frames Designed by Elastic Method . . . . .	100
5.4.2 Nonlinear Dynamic Analysis Results . . . . .	102
<b>CHAPTER 6. SPECIAL TRUSS MOMENT FRAMES (STMFs) . . . . .</b>	<b>109</b>
6.1 General . . . . .	109
6.2 AISC Design Criteria for STMFs . . . . .	110
6.2.1 Design of the Special Segment . . . . .	110
6.2.2 Design of Members outside the Special Segments (Non-Yielding Members) . . . . .	112
6.3 Overall Design Procedure . . . . .	113
6.3.1 Target Yield Mechanism . . . . .	113
6.3.2 Design of Members of Special Segment. . . . .	115
6.3.2.1 Special Segment with X-diagonal Web Members . . . . .	115

6.3.2.2 Vierendeel-Type Special Segment (without X-diagonal Web Members) . . . . .	117
6.3.2.3 Vierendeel Special Segments with Intermediate Vertical Members . . . . .	117
6.3.3 Design of Non-Yielding Elements (non-DYMs) . . . . .	120
6.3.3.1 Design of Exterior Columns with Associated Truss Girders. . . . .	124
6.3.3.2 Design of Interior Columns with Associated Truss Girders. . . . .	125
6.4 Design Examples . . . . .	126
6.4.1 Design of 9-story Ordinary STMF . . . . .	128
6.4.1.1 Design Base Shear and Lateral Force Distribution . . . . .	128
6.4.1.2 Design of Chord Members in the Special Segments . . . . .	130
6.4.1.3 Design of Non-Yielding Members outside Special Segments . . . . .	133
6.4.2 Design of 9-story Essential STMF . . . . .	138
6.4.2.1 Design Base Shear and Lateral Force Distribution . . . . .	138
6.4.2.2 Design of Chord Members in Special Segments . . . . .	141
6.4.2.3 Design of Non-Yielding Members outside the Special Segments . . . . .	142
6.5 Additional Note on Design of STMFs . . . . .	146
6.6 Verification by Nonlinear Analysis . . . . .	146
<b>CHAPTER 7. CONCENTRICALLY BRACED FRAMES (CBFs) . . . . .</b>	<b>153</b>
7.1 General . . . . .	153
7.2 AISC Design Criteria for CBFs . . . . .	154
7.3 Overall Design Procedure . . . . .	154
7.3.1 Design Base Shear . . . . .	154

7.3.2 Target Yield Mechanism . . . . .	156
7.3.3 Recommended Brace and Beam-to-Column Connection Configurations . . . . .	156
7.3.4 Design of Bracing Members . . . . .	158
7.3.4.1 Strength Criterion . . . . .	158
7.3.4.2 Fracture Criterion . . . . .	159
7.3.4.3 Compactness Criterion . . . . .	159
7.3.5 Design of Non-Yielding Members . . . . .	159
7.3.5.1 Design of Beams . . . . .	160
7.3.5.2 Design of Columns . . . . .	160
7.4 Design Examples . . . . .	163
7.4.1 Design of 3-story CBF . . . . .	167
7.4.1.1 Design Base Shear and Lateral Force Distribution . . . . .	167
7.4.1.2 Design of Braces . . . . .	170
7.4.1.3 Design of Non-Yielding Members . . . . .	171
7.4.1.3.1 Beams . . . . .	171
7.4.1.3.2 Columns . . . . .	173
7.4.2 Design of 6-story CBF . . . . .	174
7.4.2.1 Design Base Shear and Lateral Force Distribution . . . . .	174
7.4.2.2 Design of Braces . . . . .	175
7.4.2.3 Design of Non-Yielding Members . . . . .	178
7.4.2.3.1 Beams . . . . .	178
7.4.2.3.2 Columns . . . . .	179
7.5 Verification by Nonlinear Analysis . . . . .	180
7.5.1 Frames Designed by Elastic Method . . . . .	180

7.5.2 Nonlinear Analysis Results . . . . .	183
7.5.2.1 3-story CBF . . . . .	183
7.5.2.1.1 Redesign of the 3-story CBF . . . . .	184
7.5.2.2 6-story CBF . . . . .	196
<b>CHAPTER 8. TALL MOMENT FRAMES . . . . .</b>	<b>201</b>
8.1 General . . . . .	201
8.2 Design Example: 20-Story SAC Frame . . . . .	201
8.2.1 Building Geometry and Design Parameters . . . . .	201
8.2.2 Design Base Shear and Lateral Force Distribution . . . . .	204
8.2.3 Design of Beams . . . . .	205
8.2.4 Design of Columns . . . . .	205
8.3 Verification by Nonlinear Analysis . . . . .	213
8.4 Conclusion. . . . .	215
<b>CHAPTER 9. SUMMARY AND CONCLUDING REMARKS . . . . .</b>	<b>231</b>
<b>APPENDIX: THEORETICAL JUSTIFICATION . . . . .</b>	<b>235</b>
A.1 General . . . . .	235
A.2 Background . . . . .	235
A.3 Single Degree of Freedom Systems (SDOFs) . . . . .	236
A.4 Multiple Degree of Freedom Systems (MDOFs) . . . . .	239
A.5 Evaluation. . . . .	245
A.6 Conclusion . . . . .	247
<b>REFERENCES . . . . .</b>	<b>249</b>
<b>INDEX . . . . .</b>	<b>257</b>

## Foreword

---

There is no doubt that Performance-Based Seismic Design (PBSD) is an integral and important component of the future of earthquake engineering. PBSD, which started as a trend for the rehabilitation of existing structures in 1990s by the publication of Vision 2000 and FEMA-356 documents, has been extended to a viable and rational approach to the design of new structures. Many tall buildings in Los Angeles, San Francisco, and elsewhere have been or are being designed using PBSD methodology. A new generation of PBSD methodologies is currently under development by the Applied Technology Council under its ATC-58 project. Organizations such as the Los Angeles Tall Buildings Structural Design Council and the Pacific Earthquake Engineering Research Center are actively pursuing new developments and guidelines for the application of PBSD methodology for the design and evaluation of major structures.

One shortcoming of all existing PBSD approaches is that they are all basically more of an evaluation methodology than a design strategy. In other words, existing PBSD methodologies provide guidance and tools for the evaluation of seismic performance of a building that has already been designed. They do not provide clear guidance on how to design a building to achieve a desired performance. This is precisely what this book does. It provides a clear step-by-step approach that can be followed to design a building that would satisfy the desired performance given a level of seismic excitation.

The methodology presented in this book relieves the structural engineer from performing elaborate nonlinear time-history analyses during the design phase of the project and limits the application of nonlinear time-history analysis to its proper place: verification of adequate performance of an already-designed building. This is achieved by the application of simple rules of plastic design in steel, capacity-design principles, and the application of a simple static lateral force profile, which is similar to, yet somewhat different from, the static lateral force profile specified by the current prescriptive codes. The result is a building that is designed using basic engineering analysis and design techniques that performs as intended when subjected to earthquakes of specified intensity.

What makes this book exceptional is not only the fact that it explains the elegant design methodology discussed earlier, but that it applies the methodology to various structural systems via clear explanations and numerous design examples, where every step of the process is clearly defined and demonstrated.

I have no doubt that every serious practitioner of seismic design of steel structures will find this book immensely useful and practical. Teachers and students of university courses on plastic design will find this book to be a valuable teaching and learning

tool. The authors should be congratulated for their significant contribution to the art and practice of structural engineering. This is a job well done!

Farzad Naeim, Ph.D., S.E., Esq.  
Vice President and General Counsel  
John A. Martin & Associates, Inc.  
Los Angeles, CA

## Acknowledgements

---

The preparation of this volume was made possible by financial support provided by Schuff Steel Company under the leadership of Jay Allen. The book is based on recent research work carried out by the authors at The University of Michigan and sponsored by American Institute of Steel Construction; NUCOR Research and Development; and Nabih Youssef Associates, Structural Engineers.

Special thanks are due to Mohammad Reza Bayat, currently a doctoral student working with the authors, for his contribution to Chapter 8, and to Sutat Leelataviwat for his contribution to the Appendix. The authors would also like to express their sincere appreciation to the following members of the advisory group for their efforts in reviewing the manuscript and providing valuable comments and suggestions:

Abolhassan Astaneh-Asl	University of California, Berkeley
Scott Campbell	Structural Analysis Consulting Group, Louisville, KY
Tony Ghodsi	Englekirk & Sabol Engineers, Los Angeles, CA
John Hooper	Magnusson Klemencic Associates, Seattle, WA
Hamid Liaghat	Flores Lund Consultants, San Diego, CA
Farzad Naeim	John A. Martin & Associates, Los Angeles, CA
James Partridge	Smith-Emery Company, Los Angeles, CA
Derrick Roorda	De-Simone Consulting Engineers, San Francisco, CA
Arthur Ross	CYS Structural Engineers, Sacramento, CA
Bozidar Stojadinovic	University of California, Berkeley
Charles Thornton	Charles H. Thornton & Co., Easton, MD
Robert Tremblay	Ecole Polytechnique, Montreal, Canada
Nabih Youssef	Nabih Youssef & Associates, Los Angeles, CA